STRATHEDEN PLANNING PROPOSAL 2021

PP-2021-7149

TRAFFIC IMPACT ASSESSMENT

CONTENTS:

- 1. Traffic Impact Assessment (SECA Solution)
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ACN: 164611652 Ground Floor, 161 Scott Street Newcastle NSW 2300 Ph: (02) 4032-7979 Central Coast 0438 754 171 admin@secasolution.com.au

30 June 2019 P1235 BSA Stratheden Stud Rezoning

Bath Stewart Associate P O Box 403 Tamworth NSW 2340

Attn: Sonya Vickery

Dear Sonya,

Re: Traffic Impact Assessment for the proposed residential subdivision rezoning, Oxley Vale, NSW

Further to our site visit and a review of the provided documentation for the proposed rural residential subdivision located off Manilla Road, Oxley Vale, we provide the following traffic impact assessment. This assessment has been prepared in accordance with the Austroads Guidelines and Section 2.3 of the RMS Guide to Traffic Generating Developments, which provides the structure for the reporting of key issues to be addressed when determining the impacts of traffic associated with a development. This guide indicates that the use of this format and checklist ensures that the most significant matters are considered by the relevant road authority.

The report has also taken into consideration the planning requirements outlined in the Tamworth Regional Development Control Plan 2010 and the Draft Tamworth Strategic Transport Model GTA Consultants 2016.

The proposed rezoning relates to land on the corner of Manilla Road and Browns Lane, Oxley Vale as shown in Figure 1 to follow. The subject site incorporates:

- Lots 1-5 in DP 997767,
- Lots 4 and 12 in DP 245544,
- Lots 3 and 5 in DP 209387,
- Lot 4 in DP 212658.



Figure 1 – Subject site in the context of the local road network

A summary of the key issues and their comments are provided below:

Item	Comment				
Existing Situation					
2.1 Site Location and Access	The proposed rezoning relates to land located on the corner of Browns Lane and Manilla Road, Oxley Vale. The site is currently occupied by the Stratheden Stud horse agistment, with access to the site provided via an existing sealed driveway off Manilla Road and an informal access off Browns Lane.				
	There is a further access to the subject site off Manilla Road, being an unsealed driveway located to the south of the Stratheden Stud access.				
2.2.1 Road Hierarchy	Manilla Road is a state classified road (MR63) providing connection from Tamworth to areas in the north-west, including Manilla and Barraba. In the locality of the subject site it provides one lane of travel in each direction with a sealed surface in the order of 11 meters wide and no kerb or guttering. There are no street lights or pedestrian facilities, reflecting the existing limited development in the area. Manilla Road operates under the posted speed limit of 80km/hr, increasing to 100km/hr 180 metres to the north of Browns Lane.				
	Browns Lane is a local collector road with an east-west orientation and provides the main connection for local traffic accessing the state road network (Manilla Road). To the west of Manilla Road, it provides a single lane of travel in each direction with a sealed pavement in the order of 7 meters wide with widening at the intersection to allow for turning movements. No street lighting or pedestrian footpaths are provided. The posted speed limit on Browns Lane passing the subject site is 80km/hr.				
	Manilla Road connects with Browns Lane via a 4-way cross intersection, with Manilla Road having priority. There is no formal deceleration lane for left turns into Browns Lane from the north or south, however an informal shoulder provided within close proximity to Browns Lane allowing a driver to slow down and pass a turning vehicle. The width on the western leg of Browns Lane allows for 2 vehicles to hold adjacent to one another, to allow for both right and left turning movements to occur concurrently.				
	Browns Lane to the west provides an unsealed surface which also widens significantly at the intersection with Manilla Road to improve the efficiency of turning movements. There is no line marking on Browns Lane in this location.				
	Manilla Road is a state classified road under the control of RMS, whilst Browns Lane is a local road under the control of Tamworth Regional Council.				
2.2.2 Current and Proposed Roadworks, Traffic Management Works and Bikeways	No road works are currently occurring in the vicinity of the subject site. No traffic management works are currently planned in the immediate vicinity of the subject site.				
	The Draft Tamworth Strategic Transport Model completed by GTA Consultants on behalf of Tamworth Regional Council and RMS has identified the Browns Lane / Manilla Road intersection for upgrade to a roundabout as part of the future development of the land relating to this				

Item	Comment			
	assessment (Stratheden Stud), in conjunction with the ongoing development of the Hills Plain Urban Release Area to the east of Manilla Road. Upgrading of this intersection is also identified in Council's Section 94 contributions plan, with a timing of 2021 proposed.			
	To the south of the site, the NSW Government is funding upgrades to Manilla Road between Tribe and Jewry streets, Tamworth. The works involve removing two roundabouts at the Jewry Street intersection and installing new traffic lights with pedestrian crossings at both Jewry Street and Tribe Street. Work is expected to start in the first half of 2019.			
	Due to the semi-rural nature of the area, there is generally a low demand for pedestrian or cyclist movements. There is no existing cycling infrastructure in the vicinity. Council Section 94 plan includes an allowance for cycleways along Manilla Road, with a delivery between 2013-2023.			
2.3 Traffic Flows				
2.3.1 Daily Traffic Flows	 As part of the project work, Seca Solution collected traffic data at the intersection of Browns Lane and Manilla Road to determine the current road operation and traffic volumes along these roads. These surveys were completed during the afternoon (4:00pm to 6:00 pm) on Monday 25th June 2018 and the morning (7:00am to 9:15am) on Tuesday 26th June 2018, with the peak hours determined as 7:45am to 8:45am and 4:30pm to 5:30pm. A summary of the current traffic flows during the peak hours is provided below in Table 1. 			
				is provided
	Table 1 – Peak hour traffic volume	es		
	Location	Distribution	AM	РМ
	Manilla Road	Northbound	113	254
	(South of Browns Lane)	Southbound	271	157
	Browns Lane (West of Manilla Boad)	Eastbound	21	14
	Manilla Road to the south of B 384 vehicles in the AM and 4 Traffic Generating Developmen roads based on peak hour flow Manilla Road is currently oper per direction in the peak hours Browns Lane is a collector roa along its length, as well as the provides performance stand environmental capacity, with a hour and a maximum of 500 ve peak hour flows along Browns the desired capacity.	Prowns Lane has pea 11 vehicles in the PM hts provides advice or vs per direction. Under ating at LoS B with lo (maximum 271 south d providing access to main link to Manilla R dards for collector an environmental goa ehicles per hour. Tak Lane gives 32 vehicles	k two-way A. The RMS in the capac er the RMS ess than 38 inbound in t residential oad. The F streets, al of 300 vo king an ave es, which is	volumes of S Guide to ity of urban Guidelines 30 vehicles he AM). properties RMS Guide based on ehicles per rage of the well within
	of the daily traffic flows. This would indicate that the daily traffic flows along			

Item	Comment		
	Manilla Road (south of Browns Lane) are in the order of 3,975 vehicles per day, whilst daily traffic flows along Browns Lane (west of Manilla Road) are in the order of 320 vehicles per day.		
2.3.2 AADT	There is no AADT data available in the immediate locality of the subject site.		
2.3.3 Daily Traffic Flow Distribution	In the AM, traffic flows on Manilla Road are predominantly southbound with the reverse occurring in the evening reflecting local residents travelling to/from work in Tamworth.		
2.3.4 Vehicle Speeds	No speed surveys were completed as part of the survey work. Given the straight alignment of Manilla Road passing the subject site it is considered drivers would travel at or slightly above the posted speed limit.		
2.3.5 Existing Site Flows	The proposed development site is currently operating as a horse agistment and stud business, with this use generating low traffic movements. Access is currently available off both Manilla Road and Browns Lane.		
2.3.6 Heavy Vehicle Flows	There were minimal heavy vehicles recorded during the traffic surveys. In the AM peak there were 13 heavy vehicles recorded, equating to 3% of the total traffic flows through the intersection. A number of these related to school buses, including the 1 movement relating to Browns Lane.		
	In the PM peak there were 20 heavy vehicles recorded, equating to just under 5% of the total traffic flows. All of these related to through movements along Manilla Road.		
2.3.7 Current Road Network Operation	The intersection of Manilla Road and Browns Lane is well laid out, with good visibility on all approaches. Observations during the traffic surveys show that the intersection operates well with minimal delays and congestion. There are regular gaps in the flow of traffic along Manilla Road to allow for turning movements out of Browns Lane. In the case where through vehicles were required to pass a vehicle slowing to turn into Browns Lane (right in or left in) this occurred without needing to come to a complete stop, with sufficient width available to pass.		
	During the traffic surveys these movements were observed to occur infrequently consistent with the relatively low demand for turning vehicles.		
2.4 Traffic Safety and Accident History	A review of accident data provided by the RMS (Attachment B) indicates that no accidents have been recorded at the intersection of Manilla Road and Browns Lane, in the period between July 2012 and June 2017.		
	The nearest accidents were recorded along Manilla Road, with one approximately 600 metres to the south of Browns Lane relating to a B-Double travelling southbound going off road, resulting in moderate injury and a further accident 350 metres to the north of Browns Lane involving a southbound car going off road into a tree.		
	Given the good road alignment and low number of crashes it is considered that Manilla Road in the vicinity of the subject site operates in a safe and appropriate manner. The intersection of Browns Lane and Manilla Road		

Item	Comment		
	operates in a safe manner given the good visibility available for turning movements and the relatively low number of turning movements in/out of Browns Lane.		
2.5 Parking Supply and Demand			
2.5.1 On-street Parking Provision	No demand for on-street parking was observed during the site work. There is limited development in the locality, with parking demands for each lot catered for on site.		
2.5.2 Off-street Parking Provision	There is no-formal off-street parking in the general locality of the subject site. Given the size of the lots parking can be accommodated within individual holdings.		
2.5.3 Current Parking Demand and Utilisation	No demand observed for on-street parking along Manilla Road or Browns Lane.		
2.5.4 Short term set down or pick up areas	There are no formal set down or pick up areas in the vicinity of the site. A school bus was observed to use the informal verge to the north of Browns Lane (west of Manilla Road) to pick up students.		
2.6 Modal Split	Given the rural nature of the area and the proximity to services, it is considered the vast majority of commuters travel to work via private vehicle, however school bus services are provided through the locality.		
2.7 Public Transport			
2.7.1 Rail Station Locations	The nearest operating train station is in Tamworth 6.6 kilometres south- east of the subject site.		
2.7.2 Bus Stops and Associated Facilities	There are no formal bus stops in the locality along Manilla Road or Browns Lane. School services are hail and ride to meet local demands.		
2.7.3 Transport Services	There is limited public transport throughout the locality reflective of the rural nature of the area and the limited demand. Public buses are operated by Tamworth Buslines with Manilla to Tamworth Route 443 being the only service passing the subject site, providing two services in each direction per weekday with none on weekends.		
2.8 Pedestrian Network	There are no pedestrian footpaths in the locality reflective of the rural nature of the area and low pedestrian demands.		
2.9 Other Proposed Developments	A review of the Tamworth Regional Council DA Tracker shows there are no significant developments proposed in the immediate locality of the subject site.		
	There is ongoing development of the Hills Plain Urban Release Area along Browns Lane to the east of Manilla Road.		
The Development			
3.1.1 Nature of Development	The proposal allows for the rezoning of land pertaining to 815 Manilla Road, Oxley Vale and incorporating Lots 1-5 in DP 997767, Lots 4 and 12 in DP 245544, Lots 3 and 5 in DP 209387 and Lot 4 in DP 212658, to allow for residential development. The potential yield for the site will be in the		

Item	Comment			
	order of 1,100 lots, with the proposed layout plan provided in Attachment A .			
	The development will be staged and constructed over a number of years in order to meet market demands. The implementation of this staging is currently undetermined. Given the size of the development it is proposed that in the long term access to the arterial road network (Manilla Road) would be provided via two intersections being:			
	 A new intersection on Manilla Road in the location of the existing driveway access, 			
	• The intersection of Manilla Road and Browns Lane.			
	As discussed above (Section 2.2.2) Council proposes the intersection of Manilla Road / Browns Lane to be upgraded to a roundabout. The Section 94 plan identifies an indicative timeframe of 2021 with no further clarification available at this point in time.			
	These accesses have been assessed in conjunction with this proposal.			
	A detailed assessment of the access options and associated Sidra modelling is provided in this report.			
3.1.2 Access and Circulation Requirements	The layout of the subdivision will be designed in accordance with Council requirements to allow for two way vehicle movements throughout the site. All vehicles shall be able to access the external road network in a forward direction.			
3.2 Access	As discussed above vehicle access to the arterial road network will be provided via two intersections which will allow for all turning movements into and out of the site onto Manilla Road.			
	The layout of these intersections shall be dependent upon the staging of the development.			
	The proposed layout also includes access streets off Browns Lane, with these allowing for an even distribution of development flows along the internal roads for the subdivision			
3.2.1 Driveway Location	Internal driveway locations will be detailed in conjunction with individual development applications for each lot. They will be located to suit the internal site layout and will be designed in accordance with Council requirements and AS2890.			
3.2.2 Sight Distances	Sight distance for the existing Browns Lane/Manilla Road intersection has been assessed in accordance with the Austroads Guide to Road Design Part 4A. The critical requirement in this location will be Safe Intersection Sight Distance (SISD), which requires sufficient distance for a driver on the major road to observe a vehicle on the minor road and decelerate if required to avoid a collision.			
	For the posted speed limit of 80km/hr on Manilla Road a SISD of 183 metres is nominated in each direction out of Browns Lane. Manilla Road in the locality provides a straight horizontal and vertical alignment, with			

Item	Comment
	visibility to the left and right being a minimum of 250 metres. As such, sight lines at this intersection satisfy the requirements of the Austroads Guide.
	The access driveway on Manilla Road also satisfies the above sight distance requirements, with a minimum of 250 metres again available in each direction.
	Browns Lane along the northern boundary of the site provides a straight horizontal and vertical road alignment, allowing for excellent visibility along its length. For the posted speed limit of 80km/hr along Browns Lane sight distance of 183 metres is required, which can be satisfied along the length of the site frontage.
3.2.3 Service Vehicle Access	There will be occasional demands for delivery vehicles within the site and these will be able to be accommodated within the road pavement. The internal design of the roads will be in accordance with the Council DCP which will cater for the swept path requirements for waste collection vehicles and the occasional delivery vehicle.
3.2.4 Queuing at entrance to site	Allowing for the distribution of traffic across multiple access points with the majority of inbound trips to be left turns there are no vehicle queues expected for this development. Vehicles exiting the site onto Browns Lane shall be distributed across four intersections. Any queues at the Manilla Road exit shall be contained within the site.
3.2.5 Comparison with existing site access	There are two existing access driveways located off Manilla Road, with a further access off Browns Lane. The layout of these reflect typical domestic access in the locality, with widths in the order of 3 metres upon entry and gated control provided.
	The existing southern access driveway on Manilla Road will be upgraded to form a new intersection allowing access to the development. The remaining two access driveways shall not be utilised.
	New intersections shall be constructed along Browns Lane to provide access to the internal roads for the subdivision.
3.2.6 Access to Public Transport	There is limited existing public transport in the locality. The development of this land and the Hills Plain Urban Release Area may see an opportunity for bus services to be extended.
3.3 Circulation	
3.3.1 Pattern of circulation	All vehicles will be able to enter and exit the site in a forward direction and travel along the internal roadways to access the residential lots. Roadways will allow for two-way movements.
3.3.2 Internal Road width	All internal roads are to be constructed in accordance with Council's adopted Engineering Standards and Guidelines.
3.3.3 Internal Bus Movements	The future road layout of the subdivision shall be designed to accommodate public transport services in accordance with Council requirements.
3.3.4 Service Area Layout	No dedicated service area required for this type of development.

Item	Comment		
3.4 Parking			
3.4.1 Proposed Supply	The supply of parking for residences will be provided within each individual lot consistent with the DCP rate. As per typical residential subdivisions additional parking will be available on-street.		
3.4.2 Authority Parking	Tamworth Regional Council DCP provides the following parking rates for single residential dwelling houses:		
	• 2 spaces per dwelling, which shall be located behind the building line as set by Council.		
	The design for the individual lots will be subject to separate Development Applications with parking to be addressed as part of these.		
3.4.3 Parking Layout	Driveways and garages will be designed in accordance with Council DCP requirements.		
3.4.4 Parking Demand	Normal parking demands will be accommodated on site in accordance with Council DCP requirements. Parking will be provided for the residential lots as per the DCP and additional peak parking demands can be accommodated on the internal roads as required.		
3.4.5 Service Vehicle Parking	No dedicated service vehicle parking required on site. The occasional service vehicle will be able to park on the internal roads as required to service the individual dwellings within the site.		
3.4.6 Pedestrian and Bicycle Facilities	Tamworth Council Guidelines indicate a road within a residential subdivision servicing 15 lots or more must include a constructed pedestrian footpath.		
	There are no existing footpaths or cycling infrastructure in the locality reflecting its semi-rural nature. Given the proximity to the urban environment in Oxley Vale and Tamworth it is considered there will be minimal pedestrian demands external to the subdivision.		
Traffic Assessment			
4.1 Traffic Generation	Standard traffic generation rates for low density residential development of this nature are provided in the updated technical direction published by the RMS (TDT2013/04a August 2013). For regional NSW, the guide provides a peak hour rate of 0.71 vehicles per dwelling in the AM peak and 0.78 vehicles per dwelling in the PM peak with 7.4 trips per day per dwelling. For the proposed subdivision of up to 1,100 residential lots, this provides:		
	• 781 vehicle trips in the morning peak hour,		
	858 vehicle trips in the afternoon peak hour and		
	• 8,140 trips per day.		
	This allows for full development of the site, with staging subject to market demands.		
	The above vehicle trips will be distributed across the two intersections providing access to the development. Based on the site layout, the		

Item	Comment following split of development traffic has been assigned to each				
	intersection:				
	650 lots to access the site via Browns Lane (59%)				
	• 450 lots to access the site via the new southern Site Access (41%)				
	The above gives the following traffic generation inbound/outbound during the AM and PM peaks, with a standard split for residential development of 20/80 inbound/outbound applied in the AM and the reverse in the PM.				
	Manilla Road / Browns Lane:				
	• AM = 462 trips (92 inbound / 370 outbound)				
	• PM = 507 trips (406 inbound / 101 outbound)				
	Manilla Road / Site Access:				
	• AM = 320 trips (64 inbound / 256 outbound)				
	• PM = 351 trips (281 inbound / 70 outbound)				
4.1.1 Daily and Seasonal Factors	Limited daily and seasonal variation in traffic movements associated with the development, other than normal variation between workdays Monday to Friday and weekends.				
4.1.2 Pedestrian Movements	The site is not located close to shops nor within easy walking distance of educational or commercial developments. As such pedestrian demands for the development are considered to be minimal.				
4.2 Hourly distribution of trips	Typical residential development sees peak demands relating to the AM and PM peak periods, with the majority (80%) outbound in the morning with the opposite in the afternoon/evening.				
4.2.1 Origin / destinations assignment	For this assessment it has been assumed the vast majority of vehicle movements will be to/from the south of the subdivision toward Tamworth, based on the location of the subject site and minimal infrastructure and population to the north and west. There is the opportunity for commuters to travel to the east along Browns Lane to access the growing residential area, with this also providing an alternative route to schools in North Tamworth, as well as Tamworth Hospital.				
	Given the above, the following trip distribution has been allowed at the intersection of Manilla Road / Browns Lane:				
	• 80% to/from the south,				
	• 5% to/from the north and				
	• 15% to/from the east.				
	For the intersection of Manilla Road / Site access a trip distribution of:				
	• 95% to/from the south and				
	• 5% to/from the north has been applied.				

Item	Comment		
	The above distributions have been applied to the scenarios modelled, discussed in the Sidra Assessment to follow.		
4.3 Impact on Road Safety	The existing Manilla Road / Browns Lane intersection saw no accidents in the 5 year period between 2012 and 2017. This intersection provides good visibility, allowing for the safe movements of vehicles in and out of the side road (Browns Lane) and the relatively low through movements on Manilla Road.		
	The future upgrade of the Manilla Road/ Browns Lane intersection identified by Council shall provide additional capacity at this location, allowing for increased turn movements. This upgrade will be in accordance with Austroads and RMS requirements.		
	The straight alignment of Manilla Road at the proposed upgraded site access (Lot 778) will allow for suitable sight distances and the construction of an intersection in accordance with Austroads requirements. This intersection shall provide an appropriate layout to ensure road safety is maintained.		
	Given the above, it is considered that the development will have an acceptable impact on road safety.		
4.4 Impact of Generated Traffic			
4.4.1 Impact on Daily Traffic Flows	Manilla Road is a state classified road operating at Level of Service B with daily flows in the order of 3,975 vehicles per day. This road can operate with peak hour flows of up to 1400vph per direction (LoS E).		
	Allowing for peak hour flows to represent 10% of daily flows the additional hourly trips impacting Manilla Road could be in the order of 690 trips northbound in the PM. Hourly trips would therefore increase to an indicative level of 950 vehicles, below the critical LoS E. As the peak hour flows are acceptable is it concluded that the impact on daily traffic flows are within the capacity of this major urban road.		
4.4.2 Peak Hour Impacts on Intersections	The key intersections that could be impacted upon by the development are:		
	Manilla Road and Browns Lane		
	Manilla Road / Site Access		
	The operation of these intersections has been assessed using SIDRA intersection modelling as detailed below.		
4.4.3 Impact of Construction Traffic	All construction work will be located on site with minimal interaction with the local road network. There will be a requirement for construction vehicles (light and heavy) to access the site with the majority of the construction work located on the site. Construction of the new site access shall require a Construction Traffic Management Plan outlining appropriate controls which shall be prepared by the contractor in conjunction with the WAD for the project.		

Item	Comment
	The construction traffic will be less than the traffic associated with the finished development and as such is considered to have an acceptable impact upon the local road network.
4.4.4 Other Developments	There is further residential development approved to the east of the site off Browns Lane, with the proposed upgrade of Manilla Road/Browns Lane to a roundabout to accommodate ongoing development in this area.
4.5 Public Transport	
4.5.1 Options for improving services	No requirement to improve services.
4.5.2 Pedestrian Access to Bus Stops	Pedestrian access to bus stops can be provided in accordance with Council.
4.6 Recommended Works	
4.6.1 Improvements to Access and Circulation	Ensure access and internal roads / driveways are designed and constructed in accordance with Council requirements.
4.6.2 Improvements to External Road Network	The Site Access off Manilla Road shall be designed to accommodate the development flows, pending the future staging of the development. This is discussed further in the Access Assessment following.
4.6.3 Improvements to Pedestrian Facilities	Given the location of the development and its distance to shops and services there is no improvement required to pedestrian facilities along Manilla Road.
4.6.4 Effect of Recommended Works on Adjacent Developments	No works proposed that will impact on adjacent developments.
4.6.5 Effect of Recommended Works on Public Transport Services	None.
4.6.6 Provision of LATM Measures	None Required.
4.6.7 Funding	All internal site work will be funded by the developer.

Access Assessment

The key intersections of Manilla Road / Browns Lane and Manilla Road / Site Access have been assessed using SIDRA, based on the traffic data collected by Seca Solution and the traffic flows associated with the proposed development which have been calculated in Section 4.1 and assigned in accordance with Section 4.2.1 above.

As discussed in Section 3.1.1 the development will be staged, subject to market forces.

Existing Situation

Manilla Road / Browns Lane

Modelling for the existing Manilla Road / Browns Lane intersection has been undertaken to determine the current operation of this intersection. The results of the SIDRA analysis are presented below.

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)
Manilla Road	Left Turn	A/A	8.0 / 7.2	0.6./ 0.9
(Northbound)	Right Turn	A / A	7.5 / 7.1	0.0 / 0.0
Browno Lono	Left Turn	A/A	7.7 / 7.4	
Browns Lane (Westbound)	Through	A/A	7.3 / 7.5	0.4 / 0.3
	Right Turn	A / A	8.6 / 8.7	
Manilla Road (Southbound)	Left Turn	A / A	7.3 / 7.2	0.1 / 0.1
	Right Turn	A / A	7.0 / 7.5	
Browns Lane (Eastbound)	Left Turn	A/A	8.5 / 7.7	
	Through	A / A	7.4 / 7.5	0.7 / 0.4
	Right Turn	A/A	8.7 / 8.9	

Table 2 – SIDRA results, AM/PM peak existing flows 2018 at the intersection of Manilla Road / Browns Lane

As seen above the intersection currently operates at the highest LoS for all turning movements, with minimal delays and congestion.

Future Situation – Roundabout Constructed

Council has identified this intersection for upgrade to a roundabout, allowing for the increases in traffic associated with the proposed development, as well as the future development within the Hills Plain Urban Release Area.

Council's traffic modelling (Tamworth Traffic Study, completed by GTA Consultants 2016) has determined the suitable treatment for a roundabout in this location allowing for the identified developments within the area. As the level of traffic associated with the proposed Stratheden Stud northern development (approximately 600 lots) is included in this assessment it is not considered necessary to model this roundabout intersection further with this upgrade suitable to provide for **Scenario 1** being the development of the northern lands first.

An extract of the traffic study is provided to follow, with the intersection configuration shown in Figure 2 and the Sidra results shown in Figure 3.



Figure 2 – Roundabout configuration for Manilla Road / Browns Lane identified in the Tamworth Traffic Study (GTA 2016)

Table 7.2:	Manilla Road /	Browns Lane	- Intersection	Analysis Summary
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Time Period		20	020	20	030	2040			
		DOS	LOS	DOS	LOS	DOS	LOS		
4-1	AM Peak Hour	0.22	LOS A	0.24	LOS A	0.26	LOS A		
4-1	PM Peak Hour	0.22	LOS A	0.25	LOS A	0.28	LOS A		

Figure 3 – Sidra results table for Manilla Road / Browns Lane from Tamworth Traffic Study (GTA 2016)

Interim Development (prior to roundabout upgrade) – Existing Intersection + Northern Development

To determine the capacity of the **existing** intersection of Manilla Road / Browns Lane to accommodate the development of the northern lands the intersection was modelled to include the development traffic only associated with this development, with no allowance for the balance of the Hills Plain URA. Based on the split of development traffic with primary access through this intersection (outlined in Section 4.1 and assigned as per Section 4.2.1) the following distribution of traffic has been assigned through this intersection.



Figure 4 – Distribution of development traffic at Manilla Road / Browns Lane

The Sidra results for this scenario are detailed below.

Table 3 – SIDRA results,	AM/PM peak 2018	plus 650 lots at the intersection	of Manilla Road / Browns Lane
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Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)
Manilla Road	Left Turn	A/A	7.1 / 7.0	0.9/1.0
(Northbound)	Right Turn	A/A	7.6 / 7.3	0.871.0
Browno Lono	Left Turn	A/A	7.8 / 7.4	
Browns Lane	Through	A/A	7.8 / 11.0	0.9 / 3.2
(westbound)	Right Turn	A/A	9.2 / 9.2	
Manilla Road	Left Turn	A / A	7.6 / 9.6	04/20
(Southbound)	Right Turn	A/A	7.3 / 9.4	0.4 / 2.0
Browno Lono	Left Turn	A/A	9.1 / 7.8	
Browns Lane	Through	A/A	10.4 / 9.3	27.5 / 5.4
(Easidound)	Right Turn	A/A	12.3 / 11.9	

The above results show that the intersection would continue to operate at the existing LoS A with the additional traffic generated by the northern development, with negligible increases in average delays and queues for all movements.

It is noted however that the increased turning volumes would see upgrades to turn treatments required at this intersection under the Austroads Warrants. As such, despite the acceptable operation in terms of delays and queueing, the intersection requires upgrade based on the Austroads warrants with consideration of road safety implications.

Intersection Upgrade

A review of the Austroads Warrants has determined there is minimal additional capacity for turning movements into Browns Lane (west) at this intersection without triggering the need for upgrade to allow for higher treatments. This development would generate a demand for left turns into Browns Lane (west). There were 20 such movements recorded during the traffic survey. Applying the warrants an additional 15 left turns can be accommodated by the existing layout. This allows for 25 additional lots to utilise the access to Browns Lane (west) prior to upgrade.



Given this intersection is identified for upgrade to a roundabout under Council's Section 94 Contribution Plan it is not considered desirable/viable to provide for upgrades to turn treatments as a short-term solution to allow for development past 25 lots.

Once the upgrade to the intersection of Manilla Road / Browns Lane to a roundabout is undertaken, traffic associated with the northern lands (650 lots) can be accommodated.

Southern Development – Manilla Road / Site Access

Allowing for the development of the southern land the staging for this has been assessed based on the development of a primary access to Manilla Road from Lot 778.

The following scenarios have been assessed:

- 1. Operation of Manilla Road / Site Access intersection with partial development (450 lots)
- 2. Operation of Manilla Road / Site Access intersection with full development (450 lots plus northern lands accessing via Browns Lane).
- 3. Operation of Manilla Road / Site Access for future design year (2028) allowing for background growth along Manilla Road.
- 4. Operation of Manilla Road / Site Access for future design year (2028) allowing for other developments.

Operation of proposed Manilla Road / Site Access intersection, with partial development (450 lots).

This scenario allows for staging of the development to see the southern area developed prior to the northern area, with allowance for traffic generated by lots that shall have primary access via the Manilla Road / Site Access intersection. Based on the split of development traffic with primary access through this intersection (outlined in Section 4.1 and assigned as per Section 4.2.1) the following distribution of traffic has been assigned.



Figure 5 – Distribution of development traffic at Manilla Road / Site Access with partial development

The above traffic volumes were added to the existing through movements along Manilla Road and analysed using Sidra.

The road layout at the existing driveway access in this location allows no provision for through traffic to pass a turning vehicle, with no turn treatments provided and minimal shoulder width available. The required layout to accommodate the split of development traffic through the proposed Manilla Road / Site Access intersection has been assessed based on a review of the Austroads Warrants (Austroads Guide to Road Design 2017 - Part 6, Figure 2.2.6). Using the above development flows in Figure 5 it has been determined that the intersection will require minimum AUL and CHR(S) turn treatments at this intersection based on:

Left Turn: Q_L= 267, Q_M= 254 (Off chart, extrapolating would require an AUL given the high volume of left turns)



Right Turn: Q_R = 14, Q_M = 678 (Shown below \bigcirc)

Figure 6 – Austroads Warrants

The Sidra assessment has been modelled to reflect these turn treatments. The results for this scenario are detailed below.

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)	
Manilla Road	Left Turn	A/A	7.0 / 7.0	00/00	
(Northbound)	Approach	A / A	2.5 / 3.6	0.070.0	
Manilla Road	Right Turn	A/A	7.3 / 9.1	0.0/1.0	
(Southbound)	Approach	A/A	0.1 / 1.1	0.Z / 1.Z	
Site Access	Left Turn	A/A	5.2 / 5.5	00/22	
	Right Turn	A/A	7.1 / 7.5	9.072.3	

Table 4 – SIDRA results, AM/PM peak existing plus partial development 2018 at the intersection of Manilla Road / Site Access

The above results show that the intersection will operate at LoS A allowing for the traffic generated by the development.

Operation of Manilla Road / Site Access intersection with full development

This scenario is based on the premise that the intersection of Browns Lane has been upgraded to a roundabout and that the northern lands (650 lots) have been developed. The balance of the Hills Plain URA **has not** been released in this scenario. This therefore sees an increase in through movements along Manilla Road passing this southern site access associated with the northern lands traffic.



The following distribution of traffic has been assigned:



Figure 7 – Distribution of development traffic at Manilla Road / Site Access allowing for full development

It is noted that the increase in traffic flows along Manilla Road sees a full CHR turn treatment required at this intersection under the Austroads Warrants, as well as the previously identified AUL.

The Sidra results for this scenario are detailed to follow.

Table 5 – SIDRA	results AM/PM	neak existing plus	s full developme	nt 2018 at the inte	ersection of Manilla I	Road / Site Access
		pour origing plug	iun uovoiopino			

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)
Manilla Road	Left Turn	A / A	7.0 / 7.1	0.0./0.0
(Northbound)	Approach	A / A	1.7 / 2.3	0.070.0
Manilla Road	Right Turn	A/A	7.9 / 13.0	00/04
(Southbound)	Approach	A/A	0.1 / 1.5	0.272.4
	Left Turn	A/A	7.0 / 7.4	160//1
Sile Access	Right Turn	A/A	12.6 / 12.4	10.9/4.1

The above results show that the intersection will operate at LoS A allowing for the traffic generated by the whole of development. The additional through traffic for vehicles travelling to/from Browns Lane sees delays and queuing increase but remain at the highest LoS.

Operation of Manilla Road / Site Access for future design year (2028) allowing for background growth along Manilla Road

Full development flows, plus an allowance for 3% background growth per annum along Manilla Road through to 2028. Distribution as per previous Figure 7.

As per normal RMS requirements, the intersection was then assessed allowing for background growth, with a conservative 3% per annum allowed for along Manilla Road to 2028. The results of the SIDRA assessment for the future design year are shown below.



Table 6 – SIDRA results, AM/PM peak full development plus background growth to 2028 at the intersection of Manilla Road / Site Access

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)
Manilla Road	Left Turn	A / A	7.0 / 7.1	0.0 / 0.0
(Northbound)	Approach	A / A	1.4 / 2.0	0.070.0
Manilla Road	Right Turn	A/A	8.5 / 17.6	02/44
(Southbound)	Approach	A/A	0.0 / 2.2	0.3/4.4
Sita Accesa	Left Turn	A/A	12.2 / 10.4	202166
Sile Access	Right Turn	B / B	22.0 / 19.9	20.2 / 0.0

The SIDRA analysis above shows that the intersection of Manilla Road / Site Access can continue to operate to a satisfactory level of service with acceptable delays and congestion for the future design year of 2028. The turning movements out of the Site Access shall see increases in delays as a result of the high background growth applied for through movements, however these remain within acceptable limits.

Operation of Manilla Road / Site Access for future design year (2028) allowing for other developments

This scenario allows for increased traffic on Manilla Road associated with the full development of the URA. Applying the traffic scenario detailed in the Tamworth Traffic Study traffic, 1900 lots would access Manilla Road at the Browns Lane intersection (600 Stratheden lots plus 1300 Hills Plain lots).

Allowing for traffic generation in accordance with the RMS Guide and applying a split of 5% to/from north, 95% to/from south would see the following additional traffic along Manilla Road south of Browns Lane:

AM = 167 northbound / 664 southbound

PM = 730 northbound / 183 southbound



Figure 8 – Distribution of development traffic at Manilla Road / Site Access allowing for other developments

Allowing for the above distribution of traffic, with no additional background growth factor applied gives the following operation for the intersection of Manilla Road / Site Access.

Table 7 – SIDRA results, AM/PM peak proposed development plus other developments at the intersection of Manilla Road / Site Access

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)
Manilla Road	Left Turn	A / F	7.0 / 7.8	0.0 / 0.0
(Northbound)	Approach	A / F	1.1 / 2.2	0.070.0
Manilla Road	Right Turn	A/F	11.8 / 154.6	0.9./ 266.6
(Southbound)	Approach	A/F	0.1 / 83.1	0.0 / 200.0
Sita Accesa	Left Turn	F/F	2110 / 1560	006 2 / 245 0
Sile Access	Right turn	F/F	2123 / 1596	000.2 / 245.0

The SIDRA analysis above shows that the intersection of Manilla Road and the site access will fail as a result of the large increase in traffic associated with the Hills Plain development.

The Sidra assessment was then modified to accommodate these future traffic demands. The indicative intersection layout was determined as being a roundabout as shown in Figure 9 below.



Figure 9 – Required roundabout configuration at Manilla Road / Site Access to accommodate Other Developments

The high volume of through movements at this intersection associated with traffic accessing the Manilla Road / Browns Lane intersection sees demand for additional lanes on Manilla Road to separate turning movements and minimise queuing and delays to through traffic. The Sidra results are provided below.

Table 8 – SIDRA results, AM/PM peak proposed development plus other developments at the intersection of Manilla Road / Site Access with roundabout upgrade

Approach	Movement	Level of service	Average Delay (seconds)	95% Back of Queue (metres)		
Manilla Road	Left Turn	A / A	5.8 / 5.9	0.0/82.0		
(Northbound)	Approach	A/A	6.1 / 6.2	9.07 63.9		
Manilla Road	Right Turn	A/A	12.4 / 11.4	175 7 / 15 6		
(Southbound)	Approach	B / A	14.6 / 6.6	1/3.// 13.0		
	Left Turn	A / B	7.6 / 25.9	10.0 / 10.0		
Sile Access	Right turn	A/C	13.0 / 31.3	10.07 10.9		

It can be seen that with an upgraded roundabout layout the intersection shall be able to accommodate the traffic flows associated with the full development of the URA.

Separate to the capacity of the intersections to accommodate the URA and southern land development, based on the information provided in the Draft Tamworth Traffic Study, traffic volumes on Manilla Road in the vicinity of the subject site are approaching mid-block capacity. There is no indication in the S94 plan of any road duplication in this corridor.

Site Photos



Photo 1 – Cross section of Manilla Road at intersection with Browns Lane, looking north



Photo 2 - Cross section of Manilla Road at intersection with Browns Lane, looking south



Photo 3 – Typical cross section of Browns Lane (west of Manilla Road), looking east



Photo 4 – Typical cross section of Browns Lane (west of Manilla Road), looking west



Conclusion

From the above assessment and the review of the proposal and associated plans against the requirements of the RMS Guide to Traffic Generating Developments and Austroads Guide to Traffic Management, it is considered that the proposed rezoning should have no objections raised on traffic and access grounds.

The potential for additional traffic movements generated by a future 650 lot residential development associated with lands previously identified in the Tamworth Traffic Study and S94 Contribution Plan have been taken into consideration in nominated road upgrades. The opportunity to develop a further 450 lots, being the southern end of the subject site, has been assessed allowing for an upgraded site access onto Manilla Road.

The staging of the land release has been considered in several different scenarios which ultimately require the southern site access upgrade to be to a roundabout standard due to the Hills Plain URA to the north. An interim option would see various levels of turn treatment necessary to accommodate the various scenarios.

Due to the current layout of the Browns Lane/Manilla Road intersection the need for turn treatments restricts initial development to approximately 25 lots before upgrades are required. The cost of these upgrades would not be considered feasible given that the intersection is already the subject of S94 contributions and Council funding.

Low traffic flows along Browns Lane allow for the development to connect as proposed.

The rezoning can meet the requirements of the Development Control Plan in relation to traffic, parking and access as well as the overall planning for the subject site.

Please feel free to contact our office on 4032 7979, should you have any queries.

Yours sincerely,

Tyler Neve Traffic Engineer

Attached:

- A Site Plan
- B Accident Data C – Survey Data
- D Sidra Results











Attachment B RMS Accident Data MR 63 Manilla Road Tamworth Crashes 1 July 2012 to 30 June 2017





SECA solution

						Summary Crash R	eport							Centre for Ro	nspo NSW	t
# Crash Type			Contributing F	actors		Crash Mover	nent			CRASHES		2	CASUA	LTIES		2
Car Crash	1	50.0%	Speeding	0	0.0%	Intersection, adjacent approache	es	0	0.0%	Fatal	0	0.0%	Killed		0	0.0%
Light Truck Crash	0	0.0%	Fatigue	0	0.0%	Head-on (not overtaking)		0	0.0%	Serious inj.	0	0.0%	Seriously inj.		0	0.0%
Rigid Truck Crash	0	0.0%				Opposing vehicles; turning		0	0.0%	Moderate inj.	2	100.0%	Moderately inj.		2 1	00.0%
Articulated Truck Crash	1	50.0%				U-turn		0	0.0%	Minor/Other inj.	0	0.0%	Minor/Other inj.		0	0.0%
'Heavy Truck Crash	(1)	(50.0%)	Weather			Rear-end		0	0.0%	Uncategorised inj.	0	0.0%	Uncategorised in	j.	0	0.0%
Bus Crash	0	0.0%	Fine	2 10	00.0%	Lane change		0	0.0%	Non-casualty	0	0.0%	^ Unrestrained		0	0.0%
"Heavy Vehicle Crash	(1)	(50.0%)	Rain	0	0.0%	Parallel lanes; turning		0	0.0%	Self Reported Crash	0	0%	^ Belt fitted but not v	orn, No	restrai	nt
Emergency Vehicle Crash	0	0.0%	Overcast	0	0.0%	Vehicle leaving driveway		0	0.0%		-	-	inted to position on	No fieli	Ct WOI	
Motorcycle Crash	0	0.0%	Fog or mist	0	0.0%	Overtaking; same direction		0	0.0%	Time Group	% of	Dav	Crashes	C	asual	ties
Pedal Cycle Crash	0	0.0%	Other	0	0.0%	Hit parked vehicle		0	0.0%	00:01 02:59 1	50.09	612.5%	1	2016		1
Pedestrian Crash	0	0.0%	Road Surface C	onditior	ı	Hit railway train		0	0.0%	03:00 - 04:59 0	0.09	6 8 3%	1	2014		1
'Rigid or Artic. Truck "Heavy Truc	k or H	eavy Bus	Wet	0	0.0%	Hit pedestrian		0	0.0%	05:00 - 05:59 0	0.09	6 4.2%				
# mese categories are NOT mut	ally e	kciusive	Dry	2 10	00.0%	Permanent obstruction on road		0	0.0%	06:00 - 06:59 0	0.09	6 4.2%				
Location Type	•	0.00/	Snow or ice	0	0.0%	Hit animal		0	0.0%	07:00 - 07:59 0	0.09	6 4.2%				
"Intersection	0	0.0%			0.070	Off road, on straight		1	50.0%	08:00 - 08:59 0	0.09	6 4.2%				
Non Intersection	2	100.0%	Natural Ligh	ting		Off road on straight, hit object		1	50.0%	09:00 - 09:59 0	0.09	6 4.2%				
* Up to 10 metres from an intersed	tion		Dawn	0	0.0%	Out of control on straight		0	0.0%	10:00 - 10:59 0	0.09	6 4.2%				
Collision Typ	e		Davlight	1 5	50.0%	Off read on ourse, bit object		0	0.0%	11:00 - 11:59 0	0.09	6 4.2%				
Single Vehicle	2	100.0%	Dusk	0	0.0%	Out of control on curve		0	0.0%	12:00 - 12:59 0	0.09	6 4.2%				
Multi Vehicle	0	0.0%	Darkness	1 6	50.0%	Other crash type		0	0.0%	13:00 - 13:59 0	0.09	6 4.2%	McLean Period	;	% We	ek
	-	0.070	Durkness		10.070	Cread Limit		U	0.070	14:00 - 14:59 1	50.09	% 4.2%	Α	0 0	0%	17.9%
Road Classifica	tion		10 km/b or loss	•	0.00	Speed Limit	4 5	0.00/		15:00 - 15:59 0	0.09	% 4.2%	В	0	0%	7 1%
Freeway/Motorway	0	0.0%	40 km/h zono	0	0.05	6 60 km/h zone	0	0.0%		16:00 - 16:59 0	0.09	6 4.2%	c	0.	0%	17.9%
State Highway	0	0.0%	50 km/h zone	0	0.05	6 90 km/h zone	1 5	50.0%		17:00 - 17:59 0	0.09	% 4.2%	D	1 50.	0%	3.5%
Other Classified Road	2	100.0%	70 km/h zone	0	0.07	4 110 km/h zone	0	0.0%		18:00 - 18:59 0	0.09	6 4.2%	E	0.	0%	3.6%
Unclassified Road	0	0.0%	70 Kill/ll 201e	U	0.07		0	0.076		19:00 - 19:59 0	0.09	6 4.2%	F	0.0	0%	10.7%
~ 07:30-09:30 or 14:30-17:00 o	n scho	ol days	~ 40km/h or less	0 0	0.0%	~ School Travel Time Involvemer	nt	0	0.0%	20:00 - 21:59 0	0.09	6 8.3%	G	0.	0%	7.1%
			Day of the \	Neek						22:00 - 24:00 0	0.0%	% 8.3%	н	0.	0%	7.1%
Monday 0 0.0% V	Vedn	esday	0 0.0% Friday	(0.0	% Sunday 0 0.0% WE	EKEND	1	50.0%	Street Lighting Off/Nil	% of E	Dark	1	1 50.	0%	12.5%
Tuesday 1 50.0% 1	hurs	day	0 0.0% Saturday	1	50.0	% WEEKDAY 1 50.0%				1 of 1 in	Dark	100.0%	J	0.0	0%	10.7%
				#Hol	iday Pe	riods]						
New Year 0 0. Aust. Day 0 0.	0% E 0% A	Easter Anzac Day	0 0.0% (0 0.0% L	Queen's Labour [BD Day	0 0.0% Christmas 0 0.0% January SH	0	0.0% I 0.0% 、	Easter S June/Ju	SH 0 0.0% SA Ny SH 0 0.0% DA	ept./Oc ecemb	t. SH er SH	0 0.0% 0 0.0%			

Detailed Crash Report	Center for NSW
Crash No. Data Source Date Day of Week Time Distance Distance Condition Meather Speed Limit No. of Tus Travelling Street a Travelling Manoeuvre	Degree of Crash-Detailed Killed Seriously Inj. Moderately Inj. Minor/Other Inj. Uncateg'd Inj. Factors
Northern Region Tamworth Regional LGA Hallsville Manilla Rd 1103212 P 21/05/2016 Sat 14:30 350 m N BROWNS LANE 2WY STR Fine Dry 100 1 CAR M75 S in MANILLA RD 100 Proceeding in lane	MC 0 0 1 0 0
Manilla Rd Manilla Rd 1023041 P 01/04/2014 Tue 00:45 at NUMBER 766 HN 2WY STR Fine Dry 80 1 BDBL M29 S in MANILLA RD 80 Proceeding in lane E54490761 RUM 70 Off froad to left Fine Dry 80 1 BDBL M29 S in MANILLA RD 80 Proceeding in lane	MC 0 0 1 0 0
Report Totals: Crashes: 2 Fatal Crashes(FC): 0 Serious Injury Crashes(SC): 0 Moderate Injury Crashes(MC): 2 Minor/Other Injury Crashes(OC): 0 Uncategorised Injury Crashes(UC): 0 Killed(K): 0 Seriously Injured(S): 0 Moderately Injured(M): 2 Minor/Other Injured(O): 0 Uncategorised Injury Crashes(UC): 0 Crashid dataset SECA Browns Lane Manila Road Crashes 1 July 2012 to 30 June 2017 Crash self reporting, including self reported injuries began Oct 2014. Trends from 2014 are expected to vary from previous yrs. More unknowns are expected in self reported data.	Non-Casualty Crashes(NC): 0

Attachment C

Survey Data

AM Peak



Quality Traffic Advice

Intersection Peak Hour

07:45 - 08:45

	So	outhBou	ind	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Total
Vehicle Total	6	245	0	7	3	5	5	98	10	1	1	19	400
Factor	0.50	0.90	0.00	0.58	0.38	0.62	0.62	0.91	0.62	0.25	0.25	0.79	0.93
Approach Factor	0.92			0.62			0.88 0.75						

Peak Hour Vehicle Summary

Vahiala	So	outhBou	ind	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
venicie	Left	Thru	Right	Total									
Car	5	235	0	7	3	5	4	91	9	0	1	19	379
Truck	0	6	0	0	0	0	0	7	1	0	0	0	14
Bus	1	4	0	0	0	0	1	0	0	1	0	0	7

Peak Hour Pedestrians

		NE			NW			SW			SE		Total
	Left	Right	Total	Total									
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0

PM Peak





Intersection Peak Hour

16:30 - 17:30

	So	outhBou	ind	We	estboun	d	No	rthbour	nd	Ea	astboun	d	Total
	Left	Thru	Right	Total									
Vehicle Total	3	137	1	8	0	3	20	223	11	1	1	12	420
Factor	0.38	0.90	0.25	0.50	0.00	0.38	0.62	0.86	0.69	0.25	0.25	0.75	0.88
Approach Factor		0.90			0.69			0.85			0.70		

Peak Hour Vehicle Summary

Vahiala	So	outhBou	ind	We	estboun	d	No	orthbour	nd	Ea	astboun	d	Total
venicie	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	TOtal
Car	3	125	1	8	0	3	19	216	11	1	1	12	400
Truck	0	9	0	0	0	0	1	6	0	0	0	0	16
Bus	0	3	0	0	0	0	0	1	0	0	0	0	4

Peak Hour Pedestrians

		NE			NW			SW			SE		Total
	Left	Right	Total	Total									
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0

Attachment D Sidra Results

Manilla Rd / Browns Lane MOVEMENT SUMMARY

ablaSite: 1a [2018 AM Existing Browns Lane]

Manilla Road / Browns Lane Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	ance - '	Vehicl	es							
Mov	Turn	Demanc	flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: Manill	la Road										
1	L2	5	20.0	0.064	8.0	LOS A	0.1	0.6	0.09	0.08	0.09	71.4
2	T1	103	0.0	0.064	0.1	LOS A	0.1	0.6	0.09	0.08	0.09	79.1
3	R2	11	0.0	0.064	7.5	LOS A	0.1	0.6	0.09	0.08	0.09	76.4
Appro	ach	119	0.9	0.064	1.1	NA	0.1	0.6	0.09	0.08	0.09	78.5
East:	Browns	s Lane										
4	L2	7	0.0	0.016	7.7	LOS A	0.1	0.4	0.36	0.62	0.36	72.3
5	T1	3	0.0	0.016	7.3	LOS A	0.1	0.4	0.36	0.62	0.36	68.3
6	R2	5	0.0	0.016	8.6	LOS A	0.1	0.4	0.36	0.62	0.36	69.6
Appro	ach	16	0.0	0.016	7.9	LOS A	0.1	0.4	0.36	0.62	0.36	70.9
North:	Manill	a Road										
7	L2	6	16.7	0.140	7.3	LOS A	0.0	0.1	0.00	0.02	0.00	74.5
8	T1	258	4.1	0.140	0.0	LOS A	0.0	0.1	0.00	0.02	0.00	79.9
9	R2	1	0.0	0.140	7.0	LOS A	0.0	0.1	0.00	0.02	0.00	75.6
Appro	ach	265	4.4	0.140	0.2	NA	0.0	0.1	0.00	0.02	0.00	79.8
West:	Brown	s Lane										
10	L2	1	100.0	0.029	8.5	LOS A	0.1	0.7	0.38	0.67	0.38	62.4
11	T1	1	0.0	0.029	7.4	LOS A	0.1	0.7	0.38	0.67	0.38	67.8
12	R2	20	0.0	0.029	8.7	LOS A	0.1	0.7	0.38	0.67	0.38	70.8
Appro	ach	22	4.8	0.029	8.7	LOS A	0.1	0.7	0.38	0.67	0.38	70.4
All Vel	hicles	422	3.2	0.140	1.2	NA	0.1	0.7	0.06	0.09	0.06	78.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: SECA SOLUTION | Processed: Tuesday, 2 April 2019 11:51:05 AM

Project: C:\Sidra folders\P1235 Stratheden Stud Manilla Road.sip8



ablaSite: 1a [2018 PM Existing Browns Lane]

Manilla Road / Browns Lane Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performar	nce - V	Vehicl	es							
Mov	Turn	Demand F	-lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	rum	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: Manil	la Road										
1	L2	21	5.0	0.142	7.2	LOS A	0.1	0.8	0.04	0.08	0.04	75.2
2	T1	235	3.1	0.142	0.0	LOS A	0.1	0.8	0.04	0.08	0.04	79.3
3	R2	12	0.0	0.142	7.1	LOS A	0.1	0.8	0.04	0.08	0.04	76.6
Appro	ach	267	3.1	0.142	0.9	NA	0.1	0.8	0.04	0.08	0.04	78.8
East:	Browns	s Lane										
4	L2	8	0.0	0.011	7.4	LOS A	0.0	0.3	0.26	0.60	0.26	72.5
5	T1	1	0.0	0.011	7.5	LOS A	0.0	0.3	0.26	0.60	0.26	68.5
6	R2	3	0.0	0.011	8.7	LOS A	0.0	0.3	0.26	0.60	0.26	69.8
Appro	ach	13	0.0	0.011	7.7	LOS A	0.0	0.3	0.26	0.60	0.26	71.7
North:	Manill	a Road										
7	L2	3	0.0	0.081	7.2	LOS A	0.0	0.1	0.01	0.02	0.01	76.6
8	T1	144	8.8	0.081	0.0	LOS A	0.0	0.1	0.01	0.02	0.01	79.8
9	R2	1	0.0	0.081	7.5	LOS A	0.0	0.1	0.01	0.02	0.01	75.5
Appro	ach	148	8.5	0.081	0.2	NA	0.0	0.1	0.01	0.02	0.01	79.7
West:	Brown	s Lane										
10	L2	1	0.0	0.020	7.7	LOS A	0.1	0.4	0.41	0.67	0.41	67.8
11	T1	1	0.0	0.020	7.5	LOS A	0.1	0.4	0.41	0.67	0.41	67.7
12	R2	13	0.0	0.020	8.9	LOS A	0.1	0.4	0.41	0.67	0.41	70.7
Appro	ach	15	0.0	0.020	8.7	LOS A	0.1	0.4	0.41	0.67	0.41	70.4
All Ve	hicles	443	4.8	0.142	1.1	NA	0.1	0.8	0.05	0.09	0.05	78.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Sidra folders\P1235 Stratheden Stud Manilla Road.sip8

Manil Site	la			Road		Ca	/ itegory:		Brov	vns		Lane (None)
Give	way / Y	ield (Two-	Way)			•••						()
Move	ement	Performa	nce - '	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	TUITI	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	n: Manil	la Road										
1	L2	82	1.3	0.105	7.1	LOS A	0.1	0.8	0.07	0.29	0.07	74.3
2	T1	103	0.0	0.105	0.1	LOS A	0.1	0.8	0.07	0.29	0.07	77.4
3	R2	11	0.0	0.105	7.6	LOS A	0.1	0.8	0.07	0.29	0.07	74.9
Appro	bach	196	0.5	0.105	3.4	NA	0.1	0.8	0.07	0.29	0.07	76.1
East:	Browns	s Lane										
4	L2	7	0.0	0.035	7.8	LOS A	0.1	0.9	0.41	0.66	0.41	72.3
5	T1	18	0.0	0.035	7.8	LOS A	0.1	0.9	0.41	0.66	0.41	68.2
6	R2	5	0.0	0.035	9.2	LOS A	0.1	0.9	0.41	0.66	0.41	69.6
Appro	bach	31	0.0	0.035	8.1	LOS A	0.1	0.9	0.41	0.66	0.41	69.8
North	: Manill	a Road										
7	L2	6	16.7	0.144	7.6	LOS A	0.1	0.4	0.02	0.03	0.02	74.3
8	T1	258	4.1	0.144	0.0	LOS A	0.1	0.4	0.02	0.03	0.02	79.7
9	R2	6	0.0	0.144	7.3	LOS A	0.1	0.4	0.02	0.03	0.02	75.3
Appro	bach	271	4.3	0.144	0.4	NA	0.1	0.4	0.02	0.03	0.02	79.5
West	: Brown	s Lane										
10	L2	20	5.3	0.560	9.1	LOS A	3.9	27.5	0.58	0.93	0.91	64.6
11	T1	60	0.0	0.560	10.4	LOS A	3.9	27.5	0.58	0.93	0.91	65.0
12	R2	332	0.0	0.560	12.3	LOS A	3.9	27.5	0.58	0.93	0.91	68.7
Appro	bach	412	0.3	0.560	11.9	LOS A	3.9	27.5	0.58	0.93	0.91	68.1
All Ve	hicles	908	1.5	0.560	6.5	NA	3.9	27.5	0.30	0.51	0.45	73.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 1a [2018 PM Browns Lane + Development]

Manill Site	a	- 	•• •	Road		Ca	/ tegory:		Brow	vns		Lane (None)
Givew	/ay / Y	ield (Iwo-\	/Vay)									
Move	ment	Performar	nce - ˈ	Vehicl	es							
Mov ID	Turn	Demand I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
South	· Monil	ven/n	%	V/C	sec		ven	m				km/n
30utri 1	. iviariii 1.2	262	0.2	0.226	7.0		0.1	1.0	0.02	0.20	0.02	72.0
2	L2 T1	225	2.1	0.320	7.0		0.1	1.0	0.02	0.39	0.02	75.9
2	R2	200	0.0	0.326	7.3		0.1	1.0	0.02	0.39	0.02	70.3
Appro	ach	609	1.4	0.326	4.3	NA	0.1	1.0	0.02	0.39	0.02	75.1
East:	Browns	s Lane										
4	L2	8	0.0	0.132	7.4	LOS A	0.5	3.2	0.52	0.78	0.52	70.8
5	T1	65	0.0	0.132	11.0	LOS A	0.5	3.2	0.52	0.78	0.52	66.1
6	R2	3	0.0	0.132	9.2	LOS A	0.5	3.2	0.52	0.78	0.52	67.7
Appro	ach	77	0.0	0.132	10.5	LOS A	0.5	3.2	0.52	0.78	0.52	66.9
North:	Manill	a Road										
7	L2	3	0.0	0.103	9.6	LOS A	0.3	2.0	0.20	0.10	0.20	74.9
8	T1	144	8.8	0.103	0.7	LOS A	0.3	2.0	0.20	0.10	0.20	78.5
9	R2	22	0.0	0.103	9.4	LOS A	0.3	2.0	0.20	0.10	0.20	73.6
Appro	ach	169	7.5	0.103	2.0	NA	0.3	2.0	0.20	0.10	0.20	78.0
West:	Brown	s Lane										
10	L2	6	0.0	0.219	7.8	LOS A	0.8	5.4	0.57	0.84	0.58	65.5
11	T1	17	0.0	0.219	9.3	LOS A	0.8	5.4	0.57	0.84	0.58	65.4
12	R2	98	0.0	0.219	11.9	LOS A	0.8	5.4	0.57	0.84	0.58	69.0
Appro	ach	121	0.0	0.219	11.4	LOS A	0.8	5.4	0.57	0.84	0.58	68.5
All Vel	hicles	977	2.2	0.326	5.3	NA	0.8	5.4	0.16	0.43	0.16	74.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Manilla Rd / Site Access MOVEMENT SUMMARY

ablaSite: 2 [2018 AM Manilla Road/Site Access]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - '	Vehicl	es							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Manil	la Road										
1	L2	64	0.0	0.096	7.0	LOS A	0.0	0.0	0.00	0.23	0.00	73.8
2	T1	119	0.9	0.096	0.0	LOS A	0.0	0.0	0.00	0.23	0.00	77.3
Approa	ach	183	0.6	0.096	2.5	NA	0.0	0.0	0.00	0.23	0.00	76.0
North:	Manill	a Road										
8	T1	285	3.7	0.152	0.0	LOS A	0.0	0.2	0.01	0.01	0.01	79.9
9	R2	3	0.0	0.152	7.3	LOS A	0.0	0.2	0.01	0.01	0.01	60.6
Approa	ach	288	3.6	0.152	0.1	NA	0.0	0.2	0.01	0.01	0.01	79.7
West:	Site A	ccess										
10	L2	14	0.0	0.310	5.2	LOS A	1.3	9.0	0.45	0.74	0.49	53.0
12	R2	256	0.0	0.310	7.1	LOS A	1.3	9.0	0.45	0.74	0.49	58.8
Approa	ach	269	0.0	0.310	7.0	LOS A	1.3	9.0	0.45	0.74	0.49	58.6
All Veh	nicles	741	1.6	0.310	3.2	NA	1.3	9.0	0.17	0.33	0.18	69.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Sidra folders\P1235 Stratheden Stud Manilla Road.sip8



Site: 2 [2018 PM Manilla Road/Site Access]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way)

Move	ment	Performa	nce - ˈ	Vehicl	es							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Manil	la Road										
1	L2	281	0.0	0.289	7.0	LOS A	0.0	0.0	0.00	0.33	0.00	72.6
2	T1	267	0.4	0.289	0.1	LOS A	0.0	0.0	0.00	0.33	0.00	76.0
Approa	ach	548	0.2	0.289	3.6	NA	0.0	0.0	0.00	0.33	0.00	74.2
North:	Manil	la Road										
8	T1	165	7.6	0.103	0.4	LOS A	0.2	1.2	0.12	0.06	0.12	78.7
9	R2	15	0.0	0.103	9.1	LOS A	0.2	1.2	0.12	0.06	0.12	59.6
Approa	ach	180	7.0	0.103	1.1	NA	0.2	1.2	0.12	0.06	0.12	77.3
West:	Site A	ccess										
10	L2	3	0.0	0.101	5.5	LOS A	0.3	2.3	0.48	0.73	0.48	52.7
12	R2	71	0.0	0.101	7.5	LOS A	0.3	2.3	0.48	0.73	0.48	58.6
Approa	ach	74	0.0	0.101	7.4	LOS A	0.3	2.3	0.48	0.73	0.48	58.4
All Veł	nicles	802	1.7	0.289	3.4	NA	0.3	2.3	0.07	0.31	0.07	73.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Sidra folders\P1235 Stratheden Stud Manilla Road.sip8

ablaSite: 2 [2018 AM Manilla Road/Site Access with Full Development]

Manilla Road / Site Access, with full development flows allowing for through traffic associated with Browns Lane access Site Category: (None)

Giveway / Yield (Two-Way)

Move	ment	Performa	nce - ˈ	Vehicl	es							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Manil	la Road										
1	L2	64	0.0	0.135	7.0	LOS A	0.0	0.0	0.00	0.16	0.00	74.5
2	T1	196	0.5	0.135	0.0	LOS A	0.0	0.0	0.00	0.16	0.00	78.0
Approa	ach	260	0.4	0.135	1.7	NA	0.0	0.0	0.00	0.16	0.00	77.1
North:	Manill	a Road										
8	T1	597	1.8	0.312	0.0	LOS A	0.0	0.2	0.01	0.00	0.01	79.9
9	R2	3	0.0	0.312	7.9	LOS A	0.0	0.2	0.01	0.00	0.01	60.6
Approa	ach	600	1.8	0.312	0.1	NA	0.0	0.2	0.01	0.00	0.01	79.8
West:	Site A	ccess										
10	L2	14	0.0	0.499	7.0	LOS A	2.4	16.9	0.70	0.98	1.06	49.3
12	R2	256	0.0	0.499	12.6	LOS A	2.4	16.9	0.70	0.98	1.06	55.6
Approa	ach	269	0.0	0.499	12.3	LOS A	2.4	16.9	0.70	0.98	1.06	55.4
All Veł	nicles	1129	1.0	0.499	3.4	NA	2.4	16.9	0.17	0.27	0.26	71.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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ablaSite: 2 [2018 PM Manilla Road/Site Access with Full Development]

Manilla Road / Site Access, with full development flows allowing for through traffic associated with Browns Lane access Site Category: (None) Giveway / Yield (Two-Way)

Move	Movement Performance - Vehicles													
Mov	T	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID	Turn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South: Manilla		la Road												
1 L2		281	0.0	0.464	7.1	LOS A	0.0	0.0	0.00	0.21	0.00	73.9		
2	T1	609	0.2	0.464	0.1	LOS A	0.0	0.0	0.00	0.21	0.00	77.3		
Approa	ach	891	0.1	0.464	2.3	NA	0.0	0.0	0.00	0.21	0.00	76.2		
North: Manill		a Road												
8	T1	251	5.0	0.158	0.9	LOS A	0.3	2.4	0.14	0.04	0.14	78.3		
9	R2	15	0.0	0.158	13.0	LOS A	0.3	2.4	0.14	0.04	0.14	59.2		
Approa	ach	265	4.8	0.158	1.5	NA	0.3	2.4	0.14	0.04	0.14	77.3		
West:	Site A	ccess												
10	L2	3	0.0	0.184	7.4	LOS A	0.6	4.1	0.73	0.88	0.75	49.3		
12	R2	71	0.0	0.184	12.4	LOS A	0.6	4.1	0.73	0.88	0.75	55.7		
Approa	ach	74	0.0	0.184	12.2	LOS A	0.6	4.1	0.73	0.88	0.75	55.5		
All Vel	hicles	1229	1.1	0.464	2.8	NA	0.6	4.1	0.07	0.21	0.08	74.8		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 2 [2018 AM Manilla Road/Site Access with Background Growth]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South:	Manil	la Road												
1	L2	64	0.0	0.166	7.0	LOS A	0.0	0.0	0.00	0.13	0.00	74.8		
2	T1	255	0.5	0.166	0.0	LOS A	0.0	0.0	0.00	0.13	0.00	78.4		
Approa	ach	319	0.4	0.166	1.4	NA	0.0	0.0	0.00	0.13	0.00	77.6		
North: Manilla		a Road												
8	T1	776	1.8	0.405	0.0	LOS A	0.0	0.3	0.01	0.00	0.01	79.9		
9	R2	3	0.0	0.405	8.5	LOS A	0.0	0.3	0.01	0.00	0.01	60.6		
Approa	ach	779	1.8	0.405	0.0	NA	0.0	0.3	0.01	0.00	0.01	79.9		
West:	Site A	ccess												
10	L2	14	0.0	0.723	12.2	LOS A	4.0	28.2	0.87	1.21	1.79	44.0		
12	R2	256	0.0	0.723	22.0	LOS B	4.0	28.2	0.87	1.21	1.79	50.9		
Approa	ach	269	0.0	0.723	21.5	LOS B	4.0	28.2	0.87	1.21	1.79	50.6		
All Veh	nicles	1367	1.1	0.723	4.6	NA	4.0	28.2	0.17	0.27	0.36	71.5		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: SECA SOLUTION | Processed: Thursday, 30 May 2019 5:56:03 PM



Site: 2 [2018 PM Manilla Road/Site Access with Background Growth]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South:	Manil	la Road												
1	L2	281	0.0	0.558	7.1	LOS A	0.0	0.0	0.00	0.17	0.00	74.1		
2	T1	792	0.2	0.558	0.2	LOS A	0.0	0.0	0.00	0.17	0.00	77.6		
Approa	ach	1073	0.1	0.558	2.0	NA	0.0	0.0	0.00	0.17	0.00	76.7		
North: Manilla F		la Road												
8	T1	326	5.0	0.209	1.5	LOS A	0.6	4.4	0.16	0.03	0.18	77.5		
9	R2	15	0.0	0.209	17.6	LOS B	0.6	4.4	0.16	0.03	0.18	58.6		
Approa	ach	340	4.8	0.209	2.2	NA	0.6	4.4	0.16	0.03	0.18	76.8		
West:	Site A	ccess												
10	L2	3	0.0	0.295	10.4	LOS A	0.9	6.6	0.85	0.97	0.99	45.0		
12	R2	71	0.0	0.295	19.9	LOS B	0.9	6.6	0.85	0.97	0.99	51.8		
Approa	ach	74	0.0	0.295	19.5	LOS B	0.9	6.6	0.85	0.97	0.99	51.6		
All Vel	nicles	1487	1.2	0.558	2.9	NA	0.9	6.6	0.08	0.18	0.09	75.0		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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ablaSite: 2 [2028 AM Manilla Road/Site Access with Other Developments]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Move	Movement Performance - Vehicles													
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID	rum	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South: Manilla		la Road												
1	L2	64	0.0	0.225	7.0	LOS A	0.0	0.0	0.00	0.10	0.00	75.2		
2	T1	372	0.3	0.225	0.0	LOS A	0.0	0.0	0.00	0.10	0.00	78.8		
Approa	ach	436	0.2	0.225	1.1	NA	0.0	0.0	0.00	0.10	0.00	78.2		
North: Manill		la Road												
8	T1	1296	0.8	0.671	0.0	LOS A	0.1	0.8	0.01	0.00	0.01	79.9		
9	R2	3	0.0	0.671	11.8	LOS A	0.1	0.8	0.01	0.00	0.01	60.6		
Approa	ach	1299	0.8	0.671	0.1	NA	0.1	0.8	0.01	0.00	0.01	79.9		
West:	Site A	ccess												
10	L2	14	0.0	3.324	2110.2	LOS F 1	126.6	886.2	1.00	5.07	16.71	1.7		
12	R2	256	0.0	3.324	2123.7	LOS F 1	126.6	886.2	1.00	5.07	16.71	2.4		
Approa	ach	269	0.0	3.324	2123.0	LOS F11	126.6	886.2	1.00	5.07	16.71	2.4		
All Veł	nicles	2004	0.6	3.324	285.7	NA	126.6	886.2	0.14	0.70	2.25	15.3		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.

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ablaSite: 2 [2028 PM Manilla Road/Site Access with Other Developments]

Manilla Road / Site Access Site Category: (None) Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Move	Movement Performance - Vehicles													
Mov	T	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID	rum	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South: Manilla		la Road												
1	L2	281	0.0	0.858	7.8	LOS A	0.0	0.0	0.00	0.11	0.00	73.7		
2	T1	1378	0.1	0.858	1.0	LOS A	0.0	0.0	0.00	0.11	0.00	77.0		
Approa	ach	1659	0.1	0.858	2.2	NA	0.0	0.0	0.00	0.11	0.00	76.5		
North: Manilla		la Road												
8	T1	443	2.9	0.646	80.7	LOS F 11	37.2	266.6	1.00	0.04	1.44	36.4		
9	R2	15	0.0	0.646	154.6	LOS F 11	37.2	266.6	1.00	0.04	1.44	25.9		
Approa	ach	458	2.8	0.646	83.1	NA	37.2	266.6	1.00	0.04	1.44	36.1		
West:	Site A	ccess												
10	L2	3	0.0	2.656	1560.1	LOS F 11	35.1	245.8	1.00	2.32	6.54	2.2		
12	R2	71	0.0	2.656	1596.1	LOS F 11	35.1	245.8	1.00	2.32	6.54	3.1		
Approa	ach	74	0.0	2.656	1594.6	LOS F11	35.1	245.8	1.00	2.32	6.54	3.1		
All Vel	hicles	2191	0.6	2.656	72.6	NA	37.2	266.6	0.24	0.17	0.52	38.0		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

11 Level of Service is worse than the Level of Service Target specified in the Parameter Settings dialog.

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Roundabout at Manilla Road / Site Access to accommodate Other Developments



Site: 2b [2028 AM Manilla Road/Site Access with Other Developments]

Manilla Road / Site Access Site Category: (None) Roundabout Design Life Analysis (Final Year): Results for 10 years

Move	Movement Performance - Vehicles													
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID	Turri	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South:	Manil	la Road												
1	L2	64	0.0	0.049	5.8	LOS A	0.3	1.8	0.04	0.56	0.04	71.1		
2	T1	372	0.3	0.201	6.2	LOS A	1.3	9.0	0.04	0.49	0.04	72.4		
Approa	ach	436	0.2	0.201	6.1	LOS A	1.3	9.0	0.04	0.50	0.04	72.2		
North: Manill		a Road												
8	T1	1296	0.8	0.917	14.6	LOS B	24.9	175.7	1.00	0.87	1.33	65.5		
9	R2	3	0.0	0.003	12.4	LOS A	0.0	0.1	0.43	0.61	0.43	62.0		
Approa	ach	1299	0.8	0.917	14.6	LOS B	24.9	175.7	1.00	0.87	1.32	65.5		
West:	Site A	ccess												
10	L2	14	0.0	0.252	7.6	LOS A	1.4	10.0	0.53	0.73	0.53	60.8		
12	R2	256	0.0	0.252	13.0	LOS A	1.4	10.0	0.53	0.73	0.53	66.4		
Approa	ach	269	0.0	0.252	12.8	LOS A	1.4	10.0	0.53	0.73	0.53	66.2		
All Veł	nicles	2004	0.6	0.917	12.5	LOS A	24.9	175.7	0.73	0.77	0.94	67.0		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 2b [2028 PM Manilla Road/Site Access with Other Developments]

Manilla Road / Site Access Site Category: (None) Roundabout Design Life Analysis (Final Year): Results for 10 years

Move	Novement Performance - Vehicles													
Mov	T	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average		
ID	Turn	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed		
		veh/h	%	v/c	sec		veh	m				km/h		
South:	Manil	la Road												
1	L2	281	0.0	0.220	5.9	LOS A	1.3	9.0	0.10	0.54	0.10	70.8		
2	T1	1378	0.1	0.753	6.3	LOS A	12.0	83.9	0.19	0.45	0.19	71.6		
Approa	ach	1659	0.1	0.753	6.2	LOS A	12.0	83.9	0.17	0.47	0.17	71.5		
North:	Manill	a Road												
8	T1	443	2.9	0.277	6.5	LOS A	2.2	15.6	0.29	0.48	0.29	70.9		
9	R2	15	0.0	0.014	11.4	LOS A	0.1	0.5	0.26	0.62	0.26	62.8		
Approa	ach	458	2.8	0.277	6.6	LOS A	2.2	15.6	0.28	0.48	0.28	70.7		
West:	Site A	ccess												
10	L2	3	0.0	0.209	25.9	LOS B	1.6	10.9	1.00	0.95	1.00	47.5		
12	R2	71	0.0	0.209	31.3	LOS C	1.6	10.9	1.00	0.95	1.00	54.7		
Approa	ach	74	0.0	0.209	31.0	LOS C	1.6	10.9	1.00	0.95	1.00	54.5		
All Veł	nicles	2191	0.6	0.753	7.1	LOS A	12.0	83.9	0.23	0.49	0.23	70.6		

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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		PRECINCT EXTENT / BOUNDARY
		KEY INTERSECTION
	STREETS	
		ARTERIAL STATE
		PRIMARY COLLECTOR
		LOCAL COLLECTOR
		11m CARRIAGEWAY
		EXISTING CYCLEWAYS / FOOTPATHS
		FUTURE CYCLEWAYS / FOOTPATHS
	POTENTIAL F	UTURE LANDUSE
		MEDIUM DENSITY RESIDENTIAL
		SMALL LOT RESIDENTIAL
ianilla road T		LARGE LOT RESIDENTIAL
l (general Park		RECREATION / SPORT
I SPACE	FEATURES	
		CREEK / GULLY



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В	BLOCK AREAS ADDED	DAL	4.11.2021	Designed By:	DAL	AHD	\checkmark	\vee \vee		F.O.BUX 403	Emoil
Α	CONCEPTUAL ROAD LAYOUT FOR PLANNING PROPOSAL ISSUED OT COUNCIL	DAL	18.11.2020	Checked By:	DAL					Talliwolul 11377 2340	Ettiali.